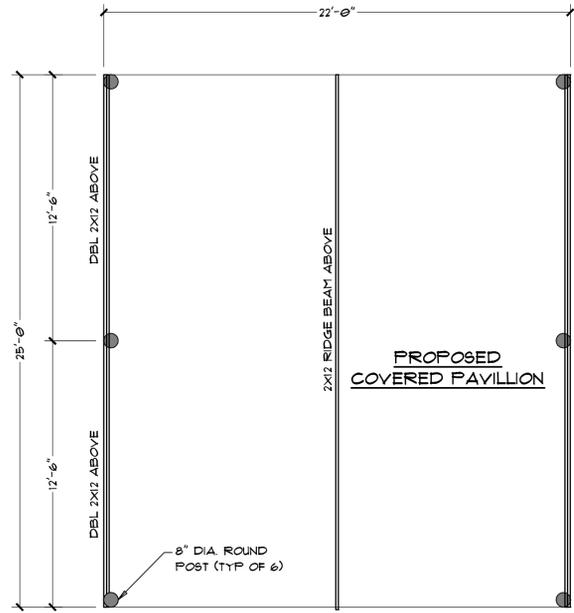


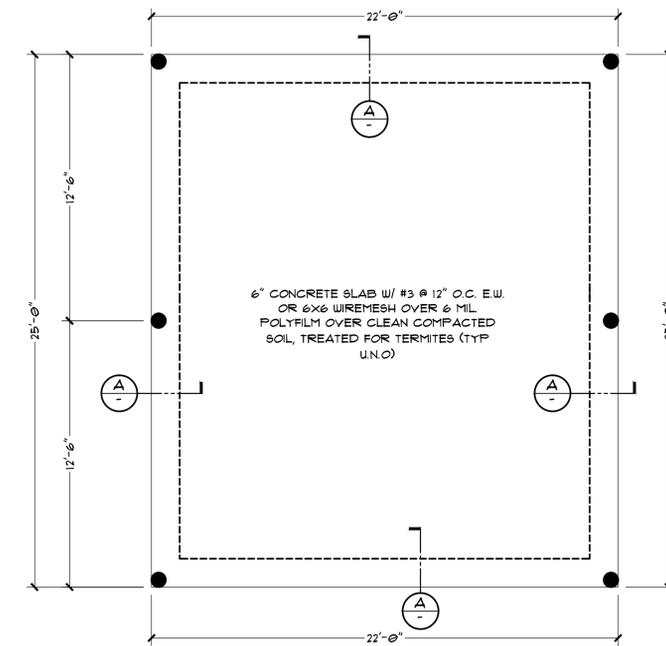
DATE: 4/14/2014 10:20:11 AM

PLOTTED BY: JEREMY LIPHAM

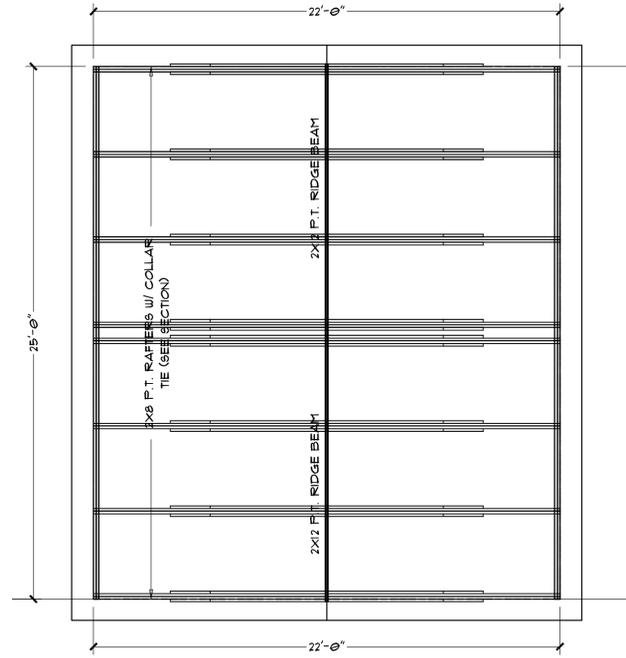
P:\1416-L.B.C.U. MODEL\1001416-L.B.C. 100.DWG



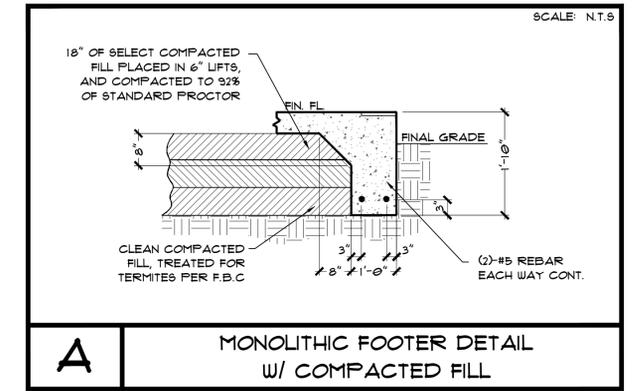
PROPOSED FLOOR PLAN
1/4" = 1'-0"



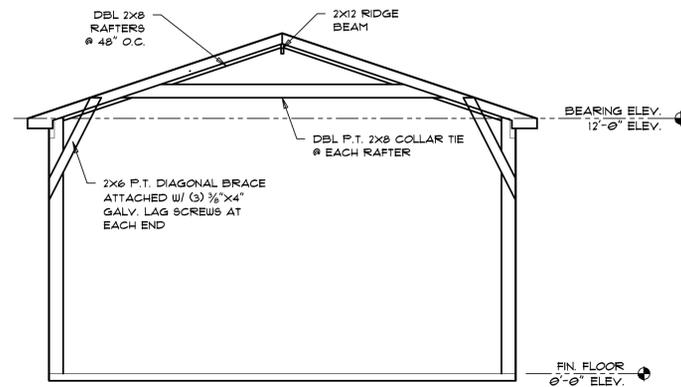
FOUNDATION PLAN
1/4" = 1'-0"



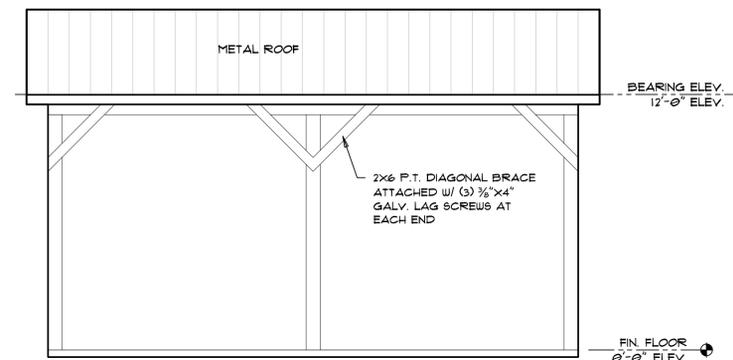
ROOF PLAN
1/4" = 1'-0"



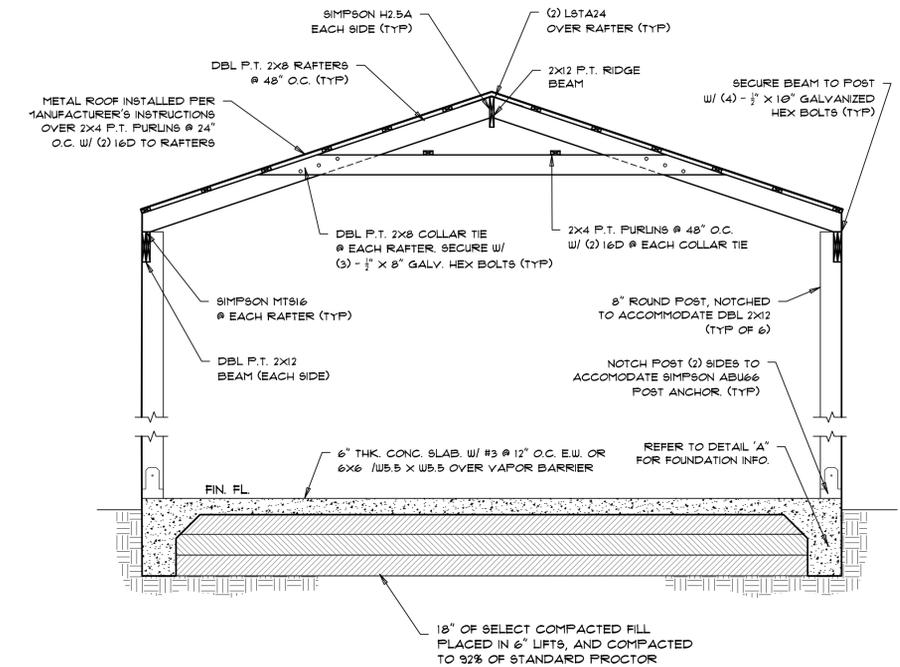
A MONOLITHIC FOOTER DETAIL
W/ COMPACTED FILL



FRONT ELEVATION
N.T.S.



RIGHT ELEVATION
N.T.S.



SECTIONAL VIEW
3/8" = 1'-0"



REV.	DATE	DRWN	CHKD	REVISION REMARKS

DRAFTSMAN: RPK	CUSTOMER: LEESBURG BOAT CLUB
CHECKED: JPL	PROJECT: 1416-L.B.C. COVERED PAVILLION
SCALE: AS SHOWN	ADDRESS: LEESBURG BOAT CLUB
DATE: 4/14/2014	CITY/STATE: LEESBURG, FL 34748
SALES: P. THOMAS	BLDG DEPT: CITY OF LEESBURG
DO NOT SCALE	

Keith E. Riddle, P.E. Professional Engineer State of Florida, Registration# 38800 115 North Canal Street Leesburg, FL 34748 PH: (352) 787-7482	ENGINEERS SEAL
PERMIT SET	

**FLOOR PLAN,
ELEVATIONS**

DRAWING NUMBER: A-1	
SHEET NUMBER: 1 OF 2	CAD FILENAME: 1416-L.B.C.100

4/14/2014

LEESBURG, FL 34748

LEESBURG BOAT CLUB

1416-L.B.C. COVERED PAVILLION

LEESBURG BOAT CLUB

A-1

DATE: 4/12/2014 10:20:15 AM PLOTTED BY: JEREMY UPHAM P:\146-L.B.C.U. MODEL\001416-L.B.C. 100.DWG

DESIGN LOADS:

(THESE DESIGN LOADS BELOW ARE ALSO INTENDED TO SERVE AS INSTRUCTIONS TO THE DELEGATED (TRUSS) ENGINEER FOR THIS PROJECT).

ROOF TRUSSES: LIVE LOAD = 20 PSF. DEAD LOAD = 17 PSF (TC) + 10 PSF (BC) (SHINGLES). FLOOR TRUSSES: LIVE LOAD = 40 PSF. DEAD LOAD = 10 PSF (TC) + 5 PSF (BC).

DESIGN WIND LOADS IN ACCORDANCE WITH SECTION R301.0 OF THE FLORIDA RESIDENTIAL BUILDING CODE 2010 EDITION, AND ASCE 7-10: WIND SPEED: VULT=140 MPH, VASD=100 MPH RISK CATEGORY II EXPOSURE CATEGORY C FULLY ENCLOSED TOPOGRAPHIC FACTOR = 1.0 WIND DIR. FACTOR KD = 0.85 GUST EFFECT, G = 0.85 DESIGN PRESSURE FOR COMPONENTS AND CLADDING = (+/-) 41.2 PSF (WORST CASE) GARAGE DOOR DESIGN PRESSURES: +22.8 PSF, -25.8 PSF

NOTE TO DELEGATED ENGINEER

PREFABRICATED WOOD JOISTS AND TRUSSES FOR ROOF ASSEMBLIES AND FLOOR SYSTEMS (IF APPLICABLE) SHALL BE DESIGNED AND MANUFACTURED IN CONFORMANCE TO ASTM D5085, ANSI/TPI 1-1995 AND WTCA 1-1995 AND SHALL BE CERTIFIED BY A FLORIDA REGISTERED ENGINEER AS EMPLOYED BY THE TRUSS MANUFACTURER (DELEGATED ENGINEER).

ALL ROOF AND FLOOR TRUSSES SHALL BE DESIGNED TO RESIST THE WORST CASE LOAD COMBINATION WHICH RESULTS IN THE MAXIMUM STRESSES BEING PLACED ON THAT COMPONENT.

GALVANIZED SHEAT-PLATES ARE TO BE ATTACHED TO EACH TRUSS AS A PROTECTIVE BARRIER, WHERE THEY BEAR ON CONCRETE OR CMU.

SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD TO FABRICATION TO ENSURE CONFORMANCE TO THE DESIGN INTENT OF THE PROJECT.

SOIL

FOOTINGS HAVE BEEN DESIGNED FOR 2000 PSF SOIL BEARING CAPACITY. ANY LESSER BEARING SHALL BE THE SOLE RESPONSIBILITY OF THE OWNER OR THE CONTRACTOR. WHERE THE SOIL BEARING CAPACITY IS NOT KNOWN OR IS IN QUESTION, THE SOIL SHALL BE TESTED BY A QUALIFIED GEOTECHNICAL ENGINEER, WHO SHALL ESTABLISH THE BEARING CAPACITY. SAID ENGINEERING SHALL COORDINATE WITH THE STRUCTURAL ENGINEER WHEN NECESSARY. COMPACTED SOILS SHALL BE TESTED TO A MINIMUM OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

SLAB ON GRADE

SLAB SHALL BE OVER 006" POLYETHYLENE VAPOR/RADON BARRIER SEALED ON TERMITE-TREATED SOIL WHICH HAS BEEN COMPACTED TO 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557, OR UNDISTURBED SOIL. SLABS SHALL BE AT A MINIMUM ELEVATION OF 8" ABOVE FINISHED GRADE. HIGHER ELEVATION MAY BE SUBSTITUTED OR REQUIRED.

REBAR EMBEDMENT AND HOOK LENGTH/BEND RADIUS:

FOR #5 REBAR FOR 90 DEG HOOK, LENGTH SHALL BE 10", MINIMUM EMBEDMENT SHALL BE 1" AND BEND RADIUS SHALL BE 2".

CONNECTORS

ALL METAL FASTENERS SHALL CONFORM TO ISANTA NER-212, AND SHALL BE GALVANIZED OR STAINLESS STEEL (WITHIN 3 MILES OF COASTAL SALE WATER AREA). FASTENERS SHALL NOT BE OVER-DRIVEN BY MORE THAN 1/8".

CONCRETE

UNLESS OTHERWISE SPECIFIED, CAST IN PLACE CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI. CAST IN PLACE GROUT 2,000 PSI, PRECAST CONCRETE LINTELS 3000 PSI AND PRE-STRESSED CONCRETE LINTELS 5000 PSI, AT 28 DAYS.

CONCRETE SHALL CONSIST OF 1" MAX AGGREGATE CONCRETE MIX WITH SLUMP BETWEEN 6" AND 11" AT THE TIME OF PLACEMENT. SEE ASTM AND ACI SPECIFICATIONS FOR ADDITIONAL CRITERIA.

CONSTRUCTION JOINTS ARE TO BE PROVIDED IN ACCORDANCE WITH THE DESIGN CODES AND GUIDELINES AT THE ENGINEER'S DIRECTION.

METHOD OF CONCRETE FORMING, PLACEMENT, AND CURING SHALL BE CONDUCTED IN ACCORDANCE WITH ACI AND ASTM SPECIFICATIONS.

CMU

ALL CONCRETE MASONRY UNITS SHALL BE STANDARD WEIGHT BLOCK CONFORMING TO ASTM C-90, TYPE II NON-MOISTURE CONTROLLED AND SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1,900 PSI.

MORTAR

MORTAR SHALL BE EITHER TYPE M OR S IN ACCORDANCE WITH ASTM C 210 AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI.

GROUT

GROUT SHALL HAVE A MINIMUM COARSE AGGREGATE SIZE OF 3/8 INCH PLACED AT AN 8" TO 10" SLUMP AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS. SEE ACI AND ASTM SPECIFICATIONS FOR ADDITIONAL CRITERIA.

ELECTRICAL

ALL WIRING PER 2010 NEC CODE WHICH INCLUDE, ARC FAULT BREAKERS, TAMPER PROOF RECEPTACLES, GFI RECEPTACLES, WEATHER PROOF RECEPTACLES, IN SERVICE COVERS, SMOKE AND CO2 DETECTOR RULES TO BE FOLLOWED.

WIRING SHALL BE 14 AWG ROMEX EXCEPT KITCHEN AND BATH RECEPTACLES.

SINGLE APPLIANCES WIRE TO BE SIZED ACCORDING TO MANUFACTURER SPECIFICATIONS. BONDING HOME TO EARTH SHALL BE ACCOMPLISHED BY A SLAB UNDERGROUND AND ONE DRIVEN COPPER CLAD 1/8" DIA. ROD, 8 FEET DEEP ALL ON #4 SOLID BARE WIRE

REINFORCING STEEL

REINFORCING STEEL SHALL BE A MINIMUM OF GRADE 40 AND SHALL CONFORM TO ASTM A615 UNLESS OTHERWISE NOTED.

ALL CONTINUOUS VERTICAL AND HORIZONTAL REBAR SHALL BE LAP SPICED WHERE NECESSARY BY WIRING TOGETHER. LAP SPICES SHALL BE CLASS B WITH A MINIMUM LAP OF 48 BAR DIAMETERS UNLESS SPECIFIED OTHERWISE.

IN LIEU OF 6" X 6" WELDED WIRE MESH OF 10 GAUGE STEEL, CONCRETE MAY BE REINFORCED WITH AN APPROVED FIBERMESH PRODUCT AND INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. FIBER MANUFACTURER MUST DOCUMENT COMPLIANCE WITH ASTM C-1116.

COVER FOR REINFORCING SHALL BE MEASURED FROM CENTER OF BAR AND BE AS FOLLOWS UNLESS NOTED OTHERWISE:

CAST IN PLACE CONCRETE AGAINST EARTH = 3" CAST IN PLACE CONCRETE EXPOSED TO EXTERIOR = 2" CAST IN PLACE CONCRETE NOT EXPOSED TO EXTERIOR = 1-1/2" GROUT FILLED MASONRY = 1-1/2" PRE-CAST AND PRE-STRESSED GROUT FILLED LINTELS = 1-1/2"

STEEL REINFORCEMENT MAY NOT BE WELDED.

WELDED WIRE MESH

WELDED WIRE MESH SHALL CONFORM TO ASTM A105. WELDED WIRE MESH SHALL BE SUPPLIED IN SHEETS NOT ROLLS.

ANCHOR BOLTS

CAST IN PLACE ANCHOR BOLTS SHALL BE GALVANIZED AND MUST EXTEND 1" MINIMUM INTO CONCRETE UNLESS OTHER NOTED.

RAILINGS

RAILINGS (IF APPLICABLE) ARE TO BE DESIGNED TO RESIST A 200 LB CONCENTRATED LOAD AT ANY POINT AND IN ANY DIRECTION.

TYPE OF LUMBER

FOR WOOD FRAME CONSTRUCTION, USE #2 SPF FOR ALL EXTERIOR AND INTERIOR BEARING WALLS. ALL ROOF FRAMING MEMBERS TO BE #2 YELLOW PINE.

TREATED LUMBER

ALL WOOD MEMBERS THAT ARE WITHIN 8" OF FINISHED GRADE LEVEL, ALL EXPOSED UNFINISHED WOOD AND ALL WOOD MEMBERS IN CONTACT WITH CONCRETE AND/OR OTHER MASONRY SHALL BE PRESERVATIVE TREATED WITH DISODIUM OCTABORATE TETRA HYDRATE TO A MINIMUM GRADE OF 0.40PCF RETENTION AND SHALL CONFORM AUPA STANDARD C1 THROUGH C23 DEPENDING ON THE APPLICATION.

ROOF DECKING

UNLESS OTHERWISE SPECIFIED, ROOF SHEATHING SHALL BE 1/2" CDX PLYWOOD INSTALLED WITH EDGE CLIPS IN EACH BAY. ALL SHEATHING SHALL BE APA RATED FOR THE USE INTENDED.

PRE-FABRICATED PRODUCTS

LAMINATED VENEER LUMBER SHALL CONFORM TO ASTM D8456 STANDARD SPECIFICATION FOR EVALUATION OF STRUCTURAL COMPOSITE LUMBER PRODUCTS. ALL WOOD STRUCTURAL PANELS, INCLUDING BUT NOT LIMITED TO PLYWOOD, OSB, WAFFER BOARD AND MEDIUM DENSITY FIBERBOARD (MDF) SHALL CONFORM TO PS-1 AND PS-2 PERFORMANCE STANDARDS FOR WOOD BASED STRUCTURAL USE PLYWOOD. MDF SHALL NOT BE USED IN ANY EXTERIOR APPLICATIONS.

THE USE OF FORMALDEHYDE-BASED GLUES AND RESINS IS DISCOURAGED. ALL FINGER JOINTED LUMBER SHALL CONFORM TO NDS SECTION 4.1.6, AND SHALL NOT BE USED IN A MANNER INCONSISTENT WITH THE LIMITATIONS OF FINGER-JOINTED LUMBER WITH RESPECT TO THE INTENDED APPLICATION.

PREFABRICATED WOOD JOISTS AND TRUSSES SHALL BE DESIGNED AND MANUFACTURED IN CONFORMANCE TO ASTM D5085, ANSI/TPI 1-1995 AND WTCA 1-1995 AND SHALL BE CERTIFIED BY A FLORIDA REGISTERED ENGINEER (DELEGATED ENGINEER).

CONTRACTOR RESPONSIBILITIES

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO FABRICATION OR START OF CONSTRUCTION. WRITTEN DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALED DIMENSIONS, ANY DEVIATIONS OR DISCREPANCIES SHALL BE PROMPTLY REPORTED TO THE ENGINEER OF RECORD. CONTRACTOR SHALL MAKE ALL EFFORTS TO PROTECT THE STRUCTURE, THE WORK PERSONS AND OTHER PEOPLE DURING CONSTRUCTION.

HE/SHE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION. THE CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND CONSTRUCTION FOR ANCHORS, EMBEDS AND SUPPORTS OR ANY OTHER ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS.

THERE SHALL NOT BE ANY CHANGES TO THESE CONSTRUCTION DOCUMENTS DURING THE DEVELOPMENT OF SHOP DRAWINGS OR DURING THE DEVELOPMENT OF SHOP DRAWINGS OR DURING CONSTRUCTION WITHOUT PRIOR WRITTEN APPROVAL BY THE ENGINEER OF RECORD.

DESIGN CRITERIA

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2010, RESIDENTIAL, CHAPTER 3, SECTION R301 DESIGN CRITERIA, AND A.S.C.E. 7.

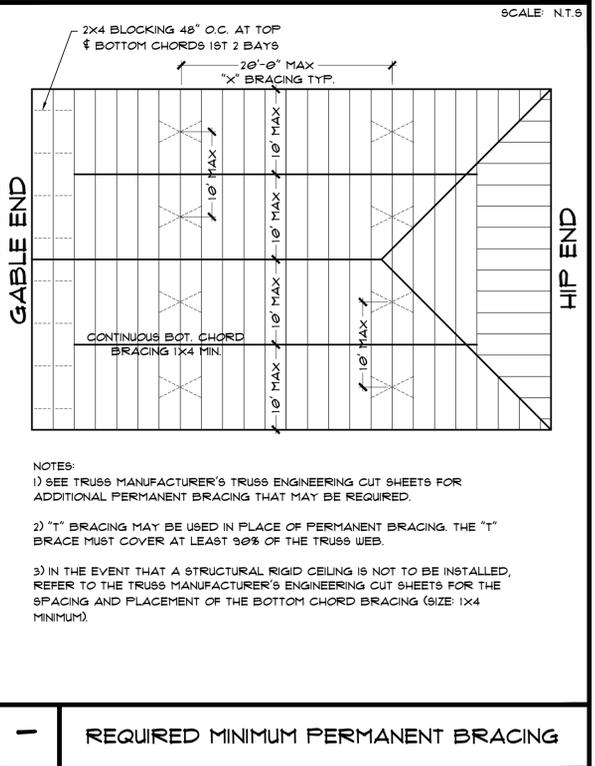
- *WIND LOAD REQUIREMENTS EMPLOYED IN THE DESIGN OF THE STRUCTURE
*ULTIMATE WIND SPEED: (3 SECOND WIND GUST) 140-MPH
*BASIC WIND SPEED: (3 SECOND WIND GUST) 108-MPH
*BUILDING CATEGORY: II - ENCLOSED
*IMPORTANCE FACTOR: 1.00
*WIND EXPOSURE: C
*DESIGN PRESSURE (WORST CASE CONDITION)
*COMPONENTS & CLADDING: +22.3-25.8 PSF
*UNIFORMLY DISTRIBUTED LIVE LOADS EMPLOYED IN THE DESIGN
*ROOF 20 PSF
*LIVING AREAS 40 PSF
*CONNECTIONS HAVE BEEN CHECKED TO WITHSTAND ALL APPLICABLE LOADS.

GENERAL NOTES

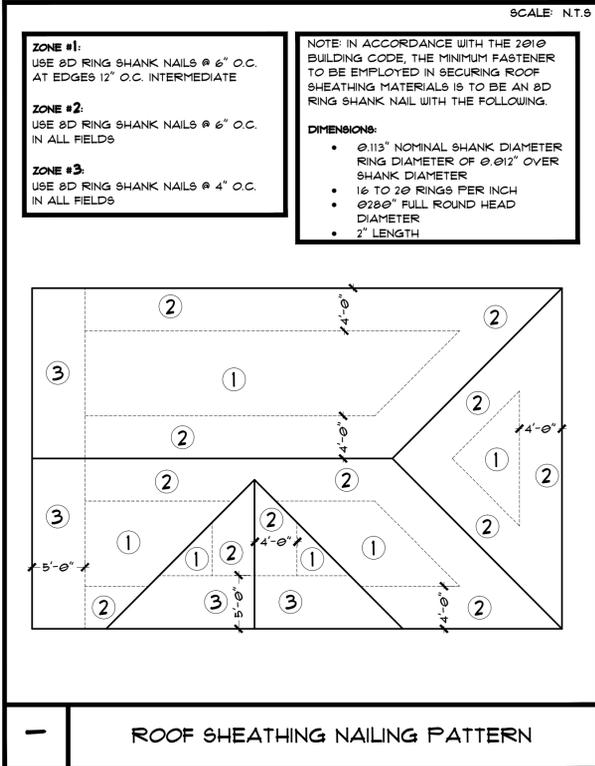
- 1. ALL CONNECTIONS HAVE BEEN CHECKED TO WITHSTAND ALL APPLICABLE LOADS.
2. ALL DETAILS AND SECTIONS SHOWN ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUCTED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT UNLESS OTHERWISE NOTED.
3. ALL CONCRETE BLOCKS SHALL BE STANDARD WEIGHT BLOCKS CONFORMING TO ASTM C90, GRADE N WITH FM=1,000 PSI, LAID IN RUNNING BOND WITH TYPE S MORTAR.
4. WHERE SHOWN, CORES OF CONCRETE BLOCK SHALL 1-#5 RE-BAR CONTINUOUS FROM FOUNDATION TO BOND BEAM AND FILLED WITH FEA GRAVEL. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI @ 28 DAYS AND SLUMP EQUAL TO 8 TO 10 INCHES.
5. IN MASONRY HEADERS, EXTEND THE BOTTOM STEEL REINFORCEMENT A MINIMUM OF 6" INTO FINISHER OR POURED COLUMN AND WIRE TIE TO VERTICAL STEEL REINFORCEMENT (REBAR).
6. SOIL BORINGS HAVE NOT BEEN MADE. FOOTINGS HAVE BEEN DESIGNED FOR A SOIL BEARING CAPACITY OF 2,000 PSF. LESSOR SOIL BEARING CAPACITY IS THE RESPONSIBILITY OF THE BUILDING CONTRACTOR.
7. FOOTINGS SHALL BEAR ON SOIL COMPACTED TO A DENSITY OF AT LEAST 95% OF THE STANDARD PROCTOR MAXIMUM DENSITY AND TREATED FOR TERMITES IN ACCORDANCE WITH THE REQUIREMENTS OF THE FLORIDA BUILDING CODE 2010.
8. FILL UNDER THE FLOOR SLAB SHALL BE COMPACTED TO AT LEAST 95% OF THE STANDARD PROCTOR MAXIMUM DENSITY AND TREATED FOR TERMITES IN ACCORDANCE WITH THE REQUIREMENTS OF THE FLORIDA BUILDING CODE 2010.
9. CONCRETE SHALL HAVE AN UNCONFIRMED COMPRESSIVE STRENGTH OF 2,500 PSI AT 28 DAYS.
10. THE BUILDING CONTRACTOR SHALL CHECK ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR OPENINGS, SLEEVES, ANCHORS, HANGERS, SLAB DEPRESSIONS, DIMENSIONS, FITCH, AND OTHER RELATED ITEMS AND SHALL ASSUME RESPONSIBILITY FOR THEIR PROPER LOCATION, PLACEMENT, AND CONTINUITY.
11. ALL REINFORCING STEEL SHALL BE NEW BILLET STEEL CONFORMING TO ASTM 615 GRADE 40.
12. ALL CONTINUOUS VERTICAL OR HORIZONTAL RE-BAR IN FOOTINGS, BEAMS, AND OTHER CONCRETE SHALL BE SPICED WHERE NECESSARY OR DESIRABLE BY WIRING TOGETHER IN CONTACT. THE LENGTH OF ALL OVERLAPS SHALL BE A MINIMUM OF 40 BAR DIAMETERS.
13. THE PRECAST LINTEL SUPPLIER SHALL DESIGN THE LINTELS FOR ADEQUATE SUPER IMPOSED LOADS, THE DESIGN SHALL MEET A.C.I. STANDARDS AND THE STANDARD BUILDING CODES.
14. ALL FRAMING SHALL BE FABRICATED AND INSTALLED AS PER A.I.T.C., T.P.I., AND THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION.
15. ALL WOOD STRUCTURAL MEMBERS SHALL BE CONTROLLED STRESS GRADE LUMBER HAVING A FIBER STRESS OF AT LEAST 1200 PSI.
16. ALL WOOD MATERIALS ARE TO BE FASTENED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2010 TABLE 2306.1 UNLESS NOTED OTHERWISE.
17. CONCRETE SLABS WILL BE REINFORCED WITH EITHER 6x6 #10 WIRE MESH OR FIBER MESH CONCRETE.
18. SPECIFIED CONNECTORS MAY BE SUBSTITUTED WITH CONNECTORS FROM OTHER MANUFACTURERS WITH EQUAL OR GREATER VALUE.

Table with columns for Model Number, Fastener Location, Allowable Uplift Loads, and Fastener Qty. Includes sub-tables for Stud Plate Ties, Embedded Truss Anchors, Hurricane Ties, Twist Straps, and Post Bases.

SIMPSON STRONG TIE CONNECTOR SCHEDULE



REQUIRED MINIMUM PERMANENT BRACING



ROOF SHEATHING NAILING PATTERN

Logo for Ro-Mac Design Center, a division of Ro-Mac Lumber & Supply. Address: 700 E. Main St., Leesburg, FL 3474 8. Phone: 352-787-4545. Fax: 352-787-9618.

Table with columns for revision: REV, DATE, DRAWN, CHKD.

Table with columns for revision: REVISION, REMARKS.

DRAFTSMAN: RPK
CHECKED: JPL
SCALE: AS SHOWN
DATE: 4/3/2014
SALES: P. THOMAS
DO NOT SCALE

CUSTOMER: LEESBURG BOAT CLUB
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CITY/STATE: LEESBURG, FL 34748
BLDG DEPT: CITY OF LEESBURG

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PERMIT SET

STRUCTURAL DETAILS
/NOTES

DRAWING NUMBER: SD-1
SHEET NUMBER: 2 OF 2
CAD FILENAME: 1416-L.B.C_100